# 'Preparation of Development Plan for Fourteen Upazilas' Package-2 

## Presentation on

Geological Works at
Ishwarganj, Raipura and Shibpur Upazila
(Draft Survey Report)

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## bjectives

To ensure the sustainable development, the prime objectives of this work is to determine subsurface soil condition of the project area and evaluating of natural geological and hydro-meteorological hazards such as earthquake, landslide and ground failure and integrate the consequence into the design of risk sensitive landuse planning.

## ield activities and ub-surface restigations

- Geomorphologic field study
- Drilling of boreholes and preparation of borehole logs;
- Collection of undisturbed and disturbed soil sample as per standard guide line;
- Conducting standard penetration tests (SPTs);
- Drilling of boreholes and casing by PVC pipe for conducting PS logging test
- Conducting Down-hole Seismic Test (PS Logging) and
- Conducting Multi-Channel Analysis of Surface Wave (MASW).


## Test Number

| Upazila Name | Borehole <br> (SPT) | Downhole <br> Seismic Test <br> (PS Logging) | MASW Test |
| :---: | :---: | :---: | :---: |
| Raipura | 12 | 3 | 5 |
| Shibpur | 20 | 3 | 5 |
| Ishwarganj | 30 | 4 | 5 |

## Drilling Locations for SPT Tes $\dagger$



## PS Logging and MASW Test Locations



## PS logging Data Acquisitions at Raipura Upazila



## PS logging Data Acquisitions at Ishwarganj Upazila



BH-03, Near Maijbagh Union Porishod, Maijbagh Union


BH-20, ChorNiclauchhoBiddaloy. IshwarganiSadar.

## PS logging Data Acquisitions at Shibpur Upazila



BH-11, Shibpur ideal school and college, ShibpurSadar


BH-04, Near Dulalpur Union porishod, Dulalpur Union

## Down-Hole Seismic (PS Logging) Test Result

-Down-Hole Seismic (PS Logging) Test data acquisition has been completed at three Upazilla in different locations on date $26{ }^{\text {th }}$ August 2016.

- Depth of observations was up to 30 meter for each hole
-Field raw data is being processed and interpreted to provide sub-surface info respect to seismic activity in the project area.



## MASW Survey at Raipura




MASW-Rai 4, Raipura College, RolastoliU Union


MASW-Ra 5 , Picikkandihtogh School, MVrzapur Union




## MASW Survey Result at Raipura Upazila



## Summary of MASW Test Results of Raipura

| MASW ID | Average Shear Wave Velocity <br> (Vs 30) |
| :---: | :---: |
| MASW Rai 1 | $163.0 \mathrm{~m} / \mathrm{s}$ |
| MASW Rai 2 | $172.7 \mathrm{~m} / \mathrm{s}$ |
| MASW Rai 3 | $159.8 \mathrm{~m} / \mathrm{s}$ |
| MASW Rai 4 | $155.2 \mathrm{~m} / \mathrm{s}$ |
| MASW Rai 5 | $162.9 \mathrm{~m} / \mathrm{s}$ |

Source: Field survey, 201
DAccording to MASW test result, the average shear wave velocities at all location are less than $180 \mathrm{~m} / \mathrm{s}$.

UShear wave velocity of the project area is showing soft to moderate soil condition for foundation.

The shear wave velocities at soil layer shows gradually increase from $110 \mathrm{~m} / \mathrm{s}$ to $230 \mathrm{~m} / \mathrm{s}$.

F From those soil velocities, it can be saying the upper soils (depth around 15 m ) are soft soil and soil hardness gradually increases by increasing depth.

## MASW Survey Result at Shibpur Upazila




## Summary of MASW Test Results of Shibpur

| MASW ID | Average Shear Wave Velocity <br> (Vs 30) |
| :---: | :---: |
| MASW Shib 1 | $188.9 \mathrm{~m} / \mathrm{s}$ |
| MASW Shib 2 | $170.9 \mathrm{~m} / \mathrm{s}$ |
| MASW Shib 3 | $178.4 \mathrm{~m} / \mathrm{s}$ |
| MASW Shib 4 | $190.3 \mathrm{~m} / \mathrm{s}$ |
| MASW Shib 5 | $205.3 \mathrm{~m} / \mathrm{s}$ |

Source:: Field survey, 201
$\square$ According to MASW test result, shear wave velocity of the project area is showing soft to moderate soil condition for foundation.
-MASW-01, MASW-04 and MASW -05 test results are showing more than $180 \mathrm{~m} / \mathrm{s}$ but others two locations the average velocity is bellow $180 \mathrm{~m} / \mathrm{s}$.

The shear wave velocities at soil layer shows gradually increase from $110 \mathrm{~m} / \mathrm{s}$ to $230 \mathrm{~m} / \mathrm{s}$.
$\square$ From those soil velocities, it can be saying the upper soils (depth around 15 m ) are soft soil and soil strength gradually increases by increasing depth.

## MASW Survey Result at Ishwarganj Upazila



S-wave veloçity cross-section


## Summary of MASW Test Results of Ishwarganj

| MASW ID | Average Shear Wave Velocity <br> $($ Vs 30) |
| :---: | :---: |
| MASW Shib 1 | $207.1 \mathrm{~m} / \mathrm{s}$ |

Source: Field survey, 201
DAccording to MASW test result, average shear wave velocity at all locations are above $180 \mathrm{~m} / \mathrm{s}$.
$\square$ From those shear wave velocity, it can be saying, the project area is showing moderate soil condition for foundation.

The shear wave velocities at soil layer shows gradually increase from $110 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$.

DFrom those soil velocities, it can be saying the upper soils (depth around 15 m ) are soft soil and soil hardness gradually increases by increasing depth.

- For geotechnical investigations, 30, 12 and 20 boreholes has been conducted at Ishwarganj, Raipura and Shibpur Upazila respectively
- The borings with SPT were carried out at all numbers of borehole in the respective Upazilas.
- Undisturbed samples and disturbed soil sample has been collected for further lab test. All samples are clearly labeled to show the project name, date, location, borehole number, depth and method of sampling; in addition, each sample should be given a serial number. Special care has been taken in the handling, transportation and storage of samples (particularly undisturbed samples) prior to testing.
- Investigation borings with standard penetration test were conducted in order to know vertical geological conditions.


## tandard enetration esting



Preparing borehole for Standard Penetration Test


Blowing with hammer for calculating Standard Penetration resistance


Drilling in the borehole


Soil sample in split spoon

## Engineering Geotechnical Logs




## xpected utoomes from the study

- Preparation of geological and geomorphologic map preparation of the study area from satellite image .
- Regional morph-tectonic and neo-tectonic mapping for potential earthquake source area identification.
- Preparation of sub-surface litho-logical 3D model of different layers through geo- technical investigation
- Foundation layer map which showing the depth of the foundation from existing ground level for footing.
- Preparation of engineering geological mapping based on AVS30
- Preparation of Seismic Hazard Assessment Map
- Peak Ground Acceleration (PGA) and Peak Ground Velocity (PGV) map.
- Recommended building height maps for both high rise building and low rise building
- Finally intensity map is prepared for high rise and low rise building


## ondusion

$\square$ All kind of field survey data( geological, geotechnical and geophysical ) acquisition has been completed.

The above mention data would give a clear idea about the geo-hazard status of particular landscape where newly urban developing activities or any other mega infrastructure project is going on and this mentioned investigation also gives idea about the vulnerability of existing build up infrastructure of a particular area.

Based on these results, proper management techniques as well as other necessary adaptation process could be addressed before or after the development activities in the studied area. It is to be mentioned that the long-term maintenance cost will be reduced and the developed structure will withstand against the potential natural hazards if the infrastructures are built following the risk informed physical land-use plan.

# Some example of 

## Final outcomes

## Geomorphology map



Preparation of geomorphologic -al maps using satellite images, borehole information and DEM

## -Valleys

- Depositional flat valley
- Wide valley
- Narrow valley
-Slopes
- Gentle slope(0-15ํ)
- Moderate slope(15은 25응
-Hillocks
- Hillock with flat top
- Hillock with narrow top
- Steep slope(>25ㅇ)


## Engineering Geology Map


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Raddish brown soft CLAY -L 1
Raddish brown stiff to hard SILT and silty clay -L 2
Raddish brown Loose to medium dense SAND with silt -L 3
Yellowish brown soft CLAY with silt -L 4
Yellowish brown stiff SILT with clay $-\mathrm{L}_{5}$ $\square$ Yellowish brown loose SAND -L 6
Raddish brown dense to very dense SAND - $\mathrm{L}_{7}$
Yellowish brown dense to very dense SAND - L 8 $\square$ Gray loose to medium dense SAND -L 11

Gray soft CLAY with trace silt -L 9
Gray stiff Silt with clay -L 10

## Engineering geological mapping based on AVS30



## Soil Type



| Ground <br> Class | $\mathrm{V}_{\mathrm{s} 30}$ | Soil Type | Relative Quality |
| :---: | :---: | :---: | :---: |
| C | $360-760 \mathrm{~m} / \mathrm{sec}$ | Very Dense/ Hard Soil and Soft rock | Very Good Soil |
| D1 | $300-360 \mathrm{~m} / \mathrm{sec}$ | Stiff / Dense to very dense/Hard Soil | Moderately good soil |
| D2 | $250-300 \mathrm{~m} / \mathrm{sec}$ | Stiff / Dense Soil | Relatively Good soil |
| D3 | $220-250 \mathrm{~m} / \mathrm{sec}$ | Medium Stiff / Medium Dense Soil | Good soil 28 |

## Foundation laver map



- Green shaded area represents zone suitable for shallow foundation to a depth of 03 meters whereas red shaded area indicates places not suitable for shallow foundation zone.
- Usually, valley areas were found to be not suitable for shallow foundation whereas the hill tops were found to be suitable for shallow foundation.


## Building Heights



## Seismic Hazard Map (Return Period 475 Years)



Peak Ground Acceleration at Ground surface

## Soil Type Map



Ground Classification Applied in this Study

| Class | Vs30 | Site Class |
| :---: | :---: | :---: |
| C | 360-760 m/sec | Very dense Soil |
| D1 | 300-360 m/sec |  |
| D2 | 250-300 m/sec |  |
| D3 | 220-250 m/sec | Stiff Soil |
| D4 | 200-220 m/sec |  |
| D5 | 180-200 m/sec |  |
| E | - $180 \mathrm{~m} / \mathrm{sec}$ | Soft Soil |

## Subsurface Lithological Layers



## Subsurface Lithological 3D Model



## Peak Ground Acceleration (PGA) Map at Ground Surface



## Identifying Geological Formation up to Depth 30m



Using Data

- Lithology

I N values of Standard Penetration Test (SPT)

- Correlation with existing Stratigraphy in and around of the study area


## Final output

| Id | Vs30 | Soil_Type | VS_Rangs | PGA_EBR | PGA_Soil | SA_0_2s_EBR | SA_0_2s_Soil | SA_1_Os_EBR | SA_1s_Soil | PP | Foundation _Depth (m) | Geomorphic_Unit |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 270 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.59 | 12 | Wide Valley,Gentle Slope, |
| 2 | ? 71 | D2 | ? $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | $\bigcirc 342$ | 0.60 | 0.684 | 0.225 | 0.3465 | 0.59 | 12 | Wide Valley,Gentle Slope, |
| 3 | 268 | 02 | $25 \mathrm{Jm} / \mathrm{stc} 30 \mathrm{cmin} / \mathrm{c}$ | J. 30 | 2.34\% | U.Ë́c | 0.684 | 0.225 | 0.3465 | 0.71 | 12 | Hillock with Flat Top,Gentle Slope, |
| 4 | 267 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.64 | 12 | Wide Valley,Hillock with Flat Top,Gentle Slope, |
| 5 | 269 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.59 | 12 | Wide Valley,Gentle Slope, |
| 6 | 271 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.58 | 12 | Gentle Slope, |
| 7 | 273 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.57 | 12 | Wide Valley,Gentle Slope, |
| 8 | 275 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.56 | 12 | Wide Valley,Gentle Slope, |
| 9 | 268 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.76 | 12 | Hillock with Flat Top, |
| 10 | 266 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.76 | 12 | Hillock with Flat Top,Gentle Slope, |
| 11 | 264 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.70 | 12 | Hillock with Flat Top,Gentle Slope, |
| 12 | 263 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.63 | 12 | Wide Valley,Hillock with Flat Top,Gentle Slope, |
| 13 | 266 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.58 | 12 | Wide Valley,Gentle Slope, |
| 14 | 271 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.56 | 12 | Gentle Slope, |
| 15 | 276 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.55 | 12 | Wide Valley,Gentle Slope, |
| 16 | 280 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.53 | 12 | Wide Valley,Gentle Slope, |
| 17 | 271 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.63 | 12 | Hillock with Flat Top,Gentle Slope, |
| 18 | 268 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.68 | 12 | Hillock with Flat Top,Gentle Slope, |
| 19 | 265 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.68 | 12 | Wide Valley,Hillock with Flat Top,Gentle Slope, |
| 20 | 260 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.64 | 12 | Wide Valley,Gentle Slope, |
| 21 | 259 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.59 | 12 | Wide Valley,Gentle Slope, |
| 22 | 264 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.56 | 12 | Wide Valley,Gentle Slope, |
| 23 | 271 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.54 | 12 | Wide Valley, Gentle Slope, |
| 24 | 278 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.52 | 12 | Wide Valley,Gentle Slope, |
| 25 | 283 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.51 | 12 | Wide Valley,Gentle Slope, |
| 26 | 262 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.39 | 8 | Narrow Valley,Gentle Slope,Depositional Flat Valley |
| 27 | 255 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.32 | 8 | Narrow Valley,Gentle Slope,Depositional Flat Valley |
| 28 | 253 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.27 | 8 | Narrow Valley,Gentle Slope, |
| 29 | 259 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.41 | 8 | Narrow Valley,Gentle Slope ${ }_{3}$ |
| 30 | 265 | D2 | $250 \mathrm{~m} / \mathrm{s}$ to $300 \mathrm{~m} / \mathrm{s}$ | 0.30 | 0.342 | 0.60 | 0.684 | 0.225 | 0.3465 | 0.56 | 8 | Narrow Valley,Gentle Slope, |

## All Geological Information at 250 m * 250 m Grid




